

The Performance of RANS Models for Prediction of Flows in Meandering Channels

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Abstract: This research work presents the prediction capability of Reynolds Averaged Navier-Stokes equations based k- ϵ & k- ω turbulence models. Two solvers (SSIIM and FLUENT) were used in this research work. The performance of the two turbulence models was gauged for one flow case. Mesh dependency check was also done. Once it was proved that both the models produce approximately same results, the k- ϵ model was then tested for its suitability for studying various flow aspects of meandering channels. Two different meandering channel geometries with the same sinuosity (centre line planform geometry), main channel meander width/floodplain width ratio and same main channel aspect ratio were used. However main channel width varied in two cases. Both bankfull and overbank flows were considered. The bend radius to main channel width ratio (r/b_c) of the wider main channel was 1.0 whereas it was 1.8 for narrow channel. The model predicted the depth averaged velocities (DAV), water surface profiles, velocity vectors in planforms at different levels with good accuracy. It captured all the salient features of the flow for inbank, low overbank and high overbank flows. From this study it can be concluded that k- ϵ model can be used with confidence in these types of meandering channels.

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1. Introduction

River flows are very complex even if the geometry of the channel is very simple such as a straight rectangular cross-section. This is mainly due to the fact that water flow is three dimensional and is derived from the combined action of a number of forces such as centrifugal forces, pressure driven forces, gravity forces and also shear forces due to transfer of momentum at the main channel floodplain interaction layers.

Flow behaviour in rivers is very important for river engineers because it is closely related to river flood prediction, warning and alleviation schemes as well as for the overall planning of river and floodplain management. It has its importance in navigation, water intakes, evolution of river bed and sediment transport. Due to all of these factors research into compound meandering channels is of much practical importance and engineering application. Research into two stage meandering compound channels has been carried out for the last three decades. This research included different approaches such as theoretical, laboratory or field observations and numerical techniques using computers.

In straight channels the secondary flows are generated due to turbulence anisotropy. The turbulence anisotropy is caused due to the presence of free surface and walls. The free surface and walls reduce the turbulence intensity normal to the surface

which is then redistributed in other directions resulting in turbulence anisotropy. Due to the existence of this anisotropy in straight channels, the standard k- ϵ or k- ω turbulence models which are based on Boussinesq's assumption does not capture the secondary circulations in these channels.

However in the case of meandering channels this turbulence anisotropy is weaker as compared to pressure and shear driven cells. The imbalance between shear driven secondary cells and centrifugal forces generated pressure driven cells are the main cause of secondary cells in meandering compound channels (Ervine et al. 1993, Wormleaton et al., 2004, Myers et al, 2000).

In the recent past there has been an increasing focus on computational fluid dynamics (CFD) for the prediction of different aspects of 3D flow fields (Wormleaton & Marriott 2007, Wormleaton & Ewunetu, 2006). The numerical approaches have a number of positive advantages over laboratory experiments including scaling, cheapness and non-intrusion. 3D numerical models have been used in particular to determine flow fields in two stage meandering channels. For this, the researchers used a number of turbulence closures to simulate flow structure (Olsen, 2011). Some authors also investigated the drag impact generated by vegetation. Some research work on meandering channel flow with trapezoidal x-sections is also available in the literature (Marriott, 1999). The

impact of turbulence closure models used in the simulation has also been investigated in different CFD studies.

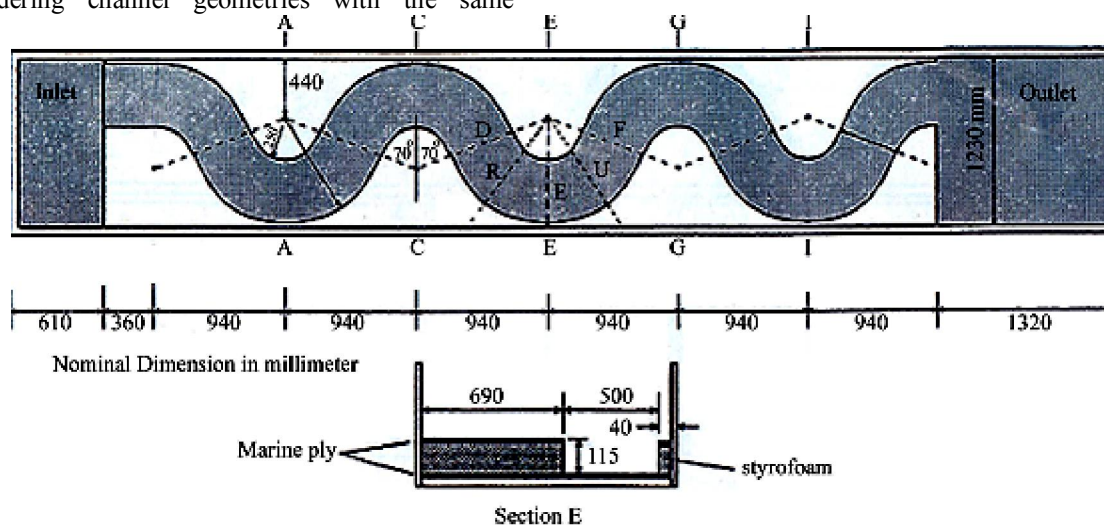
As stated above, due to weak turbulent anisotropy driven secondary cells, in this research work the k-e model has been investigated for its suitability in meandering channel simulations. In the past some researchers used a finite volume based 3D model SSIIM (Olsen, 2011) for this case. They used the k-e turbulence model with a structured mesh. However the same model can be used with an unstructured mesh generally consisting of triangular elements. It can also be used in k-omega form. In order to determine the relative performance of the different meshes a structured mesh using SSIIM was compared with a triangular mesh using FLUENT; both models using the RANS k-e model. The results were also generated with SSIIM using the k-omega model with an identical structured grid. The models were compared with each other and with Marriott's, 2007, experimental data from a compound meandering channel with a rectangular cross section. These data comprised of two main channel widths both having the same aspect ratio, sinuosity and main channel meander width/floodplain width ratio.

2. The Experimental Investigation

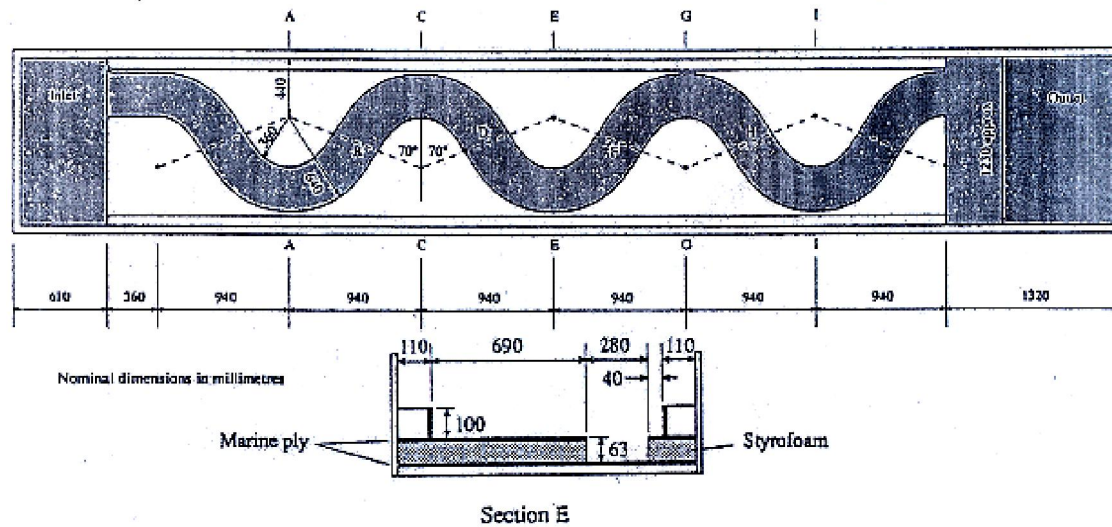
The experimental data used in this work has been obtained from Marriott's work at the University of Hertfordshire (Marriott, 1998). Two different meandering channel geometries with the same

sinuosity (centre line planform geometry), main channel meander width/floodplain width ratio and same main channel aspect ratio were used. However main channel width varied in two cases. Both bankfull and overbank flows were considered. The bend radius to main channel width ratio (r/b_c) of the wider main channel was 1.0 whereas it was 1.8 for narrow channel.

Three different bed slopes with two different overbank flow depths were considered for each geometry. The planform consisted of a meandering rectangular main channel made up of centre line circular arcs with a radius of 500 mm and included angle of 140 degrees joined tangentially at the cross over points. The resulting sinuosity was 1.3 for both the geometries. The important dimensions of the channels and floodplains along with 3D view of the channel are as shown in Fig. 1 and Table 1. Different experimental runs reported in this paper have been named by three digits. The first digit stands for geometry (1 for wide geometry and 2 for narrow geometry), the second for bed slope (1 for low, 2 for intermediate and 3 for steep slope) and the third one for bankfull and overbank flow depths (0 for bankfull, 1 for low overbank and 2 for high overbank). Thus case number MJM1 111 means wide geometry with low slope and low overbank flow depth. Table 1 shows various geometric parameters of the channels.



Planform 1 – Wide Channel



Planform 2 – Narrow Channel

Figure 1. Geometry of the experimental channels

Table 1 – Leading channel/floodplain dimensions

Dimensions (lengths in mm)	Wide Channel	Narrow Channel
Main Channel		
Width (b_c) (mm)	507	282
Depth (h_c) (mm)	115	63
Aspect Ratio	4.57:1	4.48:1
Sinuosity	1.3	1.3
r_c/b_c	1.0	1.8
Floodplain		
Width (B_f) (mm)	1230	1016
Meander belt width (B_m) (mm)	1158	938
B_m/B_f	0.94	0.92

3. The Numerical Model

3.1. Governing Equations

The governing equations for the models used in this paper are continuity and Reynolds-averaged Navier-Stokes equations (RANS). For steady state incompressible flow, the RANS equations can be written in Cartesian coordinates as (Patankar, 1980)

Continuity equation

$$\frac{\partial U_i}{\partial x_i} = 0$$

Momentum equation

$$U_j \frac{\partial}{\partial x_j} (U_i) = \frac{\nu}{\rho} \frac{\partial}{\partial x_j} \left(\frac{\partial U_i}{\partial x_j} + \frac{\partial U_j}{\partial x_i} \right) - \frac{1}{\rho} \frac{\partial P}{\partial x_i} + F_i + (-\overline{u_i u_j})$$

The terms in these equations are as follows. P is the pressure, ν and ρ are the kinematic viscosity and density of the water respectively, U_i is the time-averaged velocity component in x_i direction, F_i denotes external force (gravity forces), and $u_i u_j$ are the Reynolds stresses which result from the decomposition of instantaneous velocities into their mean and fluctuating components. In these models the Reynolds stresses are expressed using the Boussinesq relationship as below.

$$-\overline{u_i u_j} = \nu_t \left(\frac{\partial U_i}{\partial x_j} + \frac{\partial U_j}{\partial x_i} \right) - \frac{2}{3} k \delta_{ij}$$

where ν_t is eddy viscosity, k and δ_{ij} are turbulent kinetic energy and Kronecker delta

respectively. For the k- ϵ model, transport equations are derived for the turbulent kinetic energy, k , and the rate of viscous dissipation, ϵ . These are then solved and used to calculate the eddy viscosity in above equation as $\nu_t = C_\mu k^2/\epsilon$, where C_μ is a constant in the model which takes a standard value of 0.09. The k- ω model utilizes transport equations for k and the turbulence frequency $\omega = \epsilon/k$ from which ν_t can be determined.

In all cases a logarithmic wall law for rough boundaries was used to determine near-wall velocity. This is written as:

$$\frac{U}{u_*} = \frac{1}{\kappa} \ln \left(\frac{30y_b}{k_s} \right)$$

U is the velocity at a distance y_b from the boundary, u_* is the shear velocity, k_s is an equivalent particle diameter for the bed material and κ is the von Karman constant (0.4). The near-wall values of k and epsilon are taken as:

$$k = \frac{u_*^2}{\sqrt{C_\mu}} \quad \text{and} \quad \epsilon = \frac{u_*^3}{\kappa y_b}$$

The equivalent particle diameters for each run were calculated from Marriott's measured values of Manning's n using Strickler's formula

A "rigid lid" assumption, which prescribes a surface that is usually planar was taken as a boundary condition for the free surface (That is water surface was defined as a plane of symmetry). At the plane of symmetry, normal gradients of all the variables and normal velocity were set as zero except the rate of dissipation. The pressure on the surface was not set at zero but represents the variation in the water depth

that would occur if the surface was not fixed. Thus a pressure greater than zero represents super elevation and a pressure less than zero represents surface depression. No slip wall boundary condition was taken at bed and side walls.

3.2. Grid Design and Dependence

A structured grid covering four meander wavelengths was adopted for the SSIIM modelling. Each wavelength was modelled using 40 x-increments. There were 40 lateral y-increments divided equally between the channel and floodplains. The main channel depth was modelled using twelve depth z-increments. The meandering channel geometry was created by blocking out the bottom seven of these increments in order to create the floodplains. The size of the lateral floodplain increments increased geometrically from the main channel interface to the outer floodplain edge. Thus the increments were small in the high shear main channel/floodplain interface region so as to capture the relatively rapid local flow variations.

An unstructured tetrahedral mesh was used to discretize the geometry for simulation using FLUENT. The mesh used had the following node numbers $201 \times 15 \times 10$ in the longitudinal, lateral and vertical directions respectively in the main channel. Since flow parameters within and close to the channel boundaries show the greatest local variation, the mesh was made fine in these areas and gradually coarsened on the floodplain away from the main channel. As a result a mesh of $201 \times 36 \times 6$ was used in the floodplain. The lateral grid sizes on the floodplains decreased gradually from the outer floodplain edge to the main channel bank. The final mesh used for simulation in FLUENT has been shown in Fig.2.

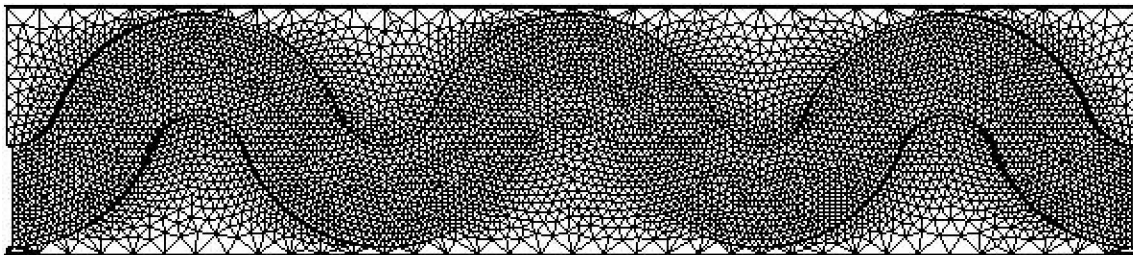


Figure 2. The unstructured FLUENT mesh

A post processing check on mesh quality based on assessing the skewness of the generated mesh elements indicated that the mesh is of high quality and would not compromise solution stability. The solutions at successive meander apices were compared in all the models and the differences were sufficiently small to assume with confidence that

flow parameters within the observed meander were unaffected by inlet or outlet conditions.

4. Results and Discussion

4.1. Model Comparisons

The SSIIM grid dependence was checked by comparison with the FLUENT results and also by comparing the results for one typical run (no.111)

with those obtained by doubling the number of x, y and z nodes. The mean absolute error (MAE) as a percentage of the velocities in the x, y and z directions were 1.9%, 2.9% and 4.3% respectively. These were thought to be acceptably small bearing in mind the accuracy of the measured data.

The FLUENT model improved the overall percentage MAE for primary velocities in run 111 by 0.3% over the SSIIM solutions. In view of probable interpolation and numerical computational errors, this difference was accepted by the Authors as additional confirmation of the grid independence of both models.

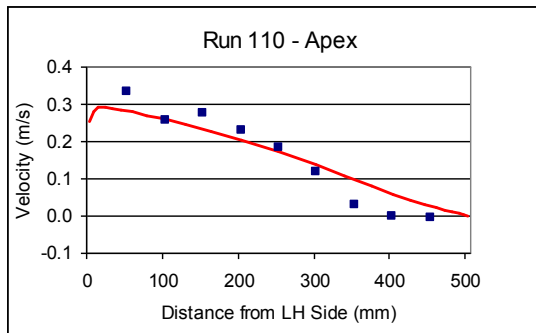
It is well known that the k-e model tends to under-estimate the extent of separation and re-circulation at curved walls due to its tendency to over-predict the magnitude of turbulent shear stresses in these regions. The k-omega model on the other hand gives less turbulent diffusion than the k-e model (Olsen, 2011) and so may be better suited to these regions. The authors compared the performance of the SSIIM k-e and k-omega models for inbank run 110. It was found that the k-omega model was faster and more robust, i.e. allowed the use of larger relaxation factors, than the k-e method. It was

observed that over the whole meander the k-omega model was 1% better in terms of the percentage MAE for primary velocities than the k-e model. However the k-omega model was 1.7% better in the cross-over sections where the re-circulation was greatest. In areas of no re-circulation the difference in the models was less than 0.5%. Both models under-predicted the extent of the re-circulation zone.

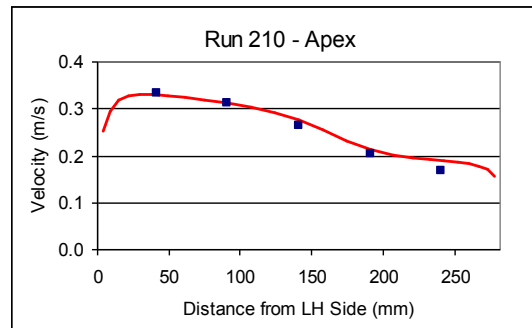
4.2. Bankfull Runs

The variation in depth averaged primary velocities for the in-bank runs in the two channel geometries are shown in Figure 3. It can be seen from Figure 3 that the calculated depth averaged primary velocities are very similar to the observed values for both channels at the apex showing maximum values near the inner side. However at the cross-overs the velocities at the inner side are over-predicted in both cases as discussed above.

Figure 4 shows the planform variation in velocities at 45mm above the bed. As reported by Marriott (1998 & 1999) and Wormleaton & Marriott (2007) separation and recirculation is observed downstream of the inner apex wall in the wide channel (run 110) but not in the narrow channel (run 210).

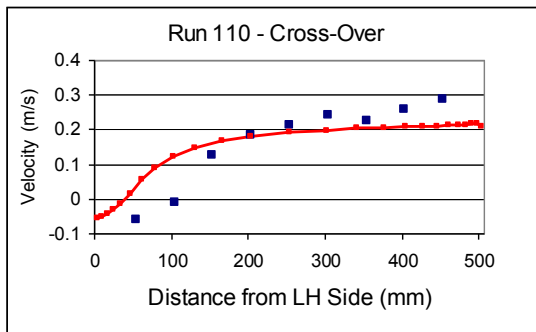


(a)



(b)

Figure 3(a-b). Depth averaged velocities for in-bank flows (square indicates observed and line shows calculated values of velocities)

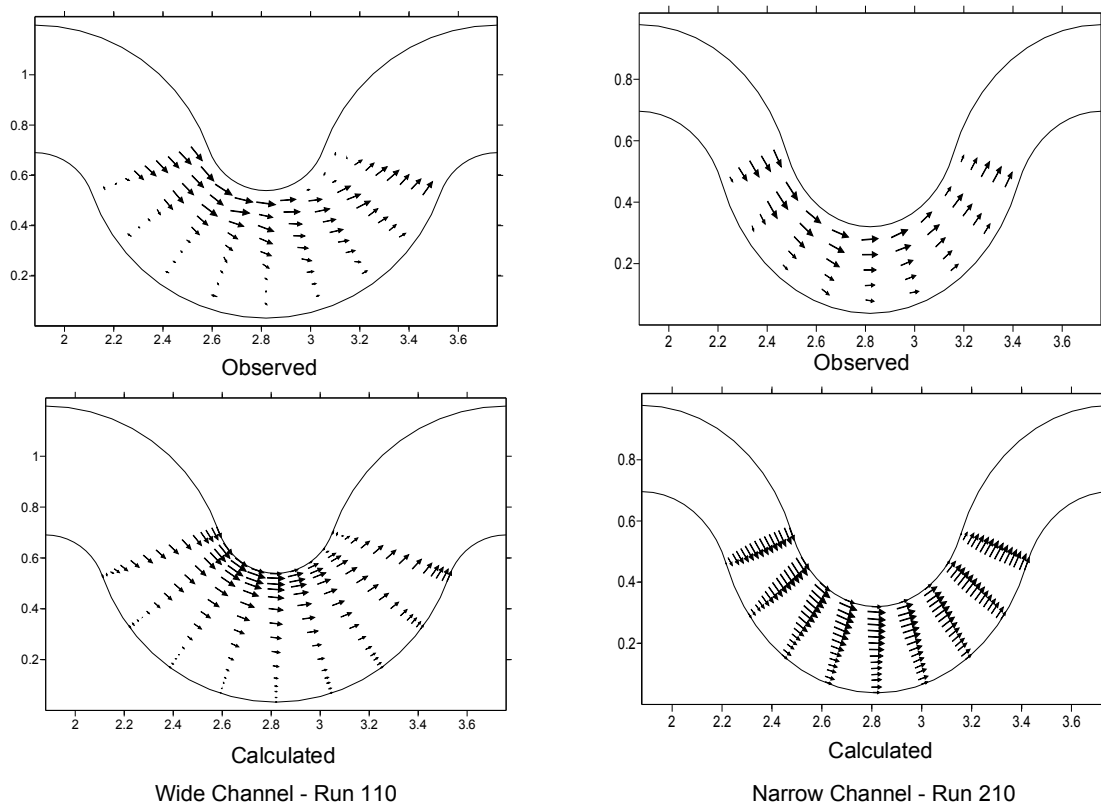


3(c)



3(d)

Figure 3 (c-d). Depth averaged velocities for in-bank flows (square indicates observed and line shows calculated values of velocities)



Note: All horizontal scales are distance downstream in metres
 All vertical scales are lateral distance in metres.
 Velocity scale 0.3m/s = ↑

Figure 4 - Planform velocity distributions for bankfull flows - Wide channel at 50mm above the bed and narrow channel at 45mm above the bed.

Bagnold (1960) stated that this separation occurs when the ratio of the channel centre-line radius (r_c) to the channel width (b_c) is less than 2. The value of this ratio (r_c/b_c) is 1.0 for the wide channel and 1.8 for the narrow channel. Thus the observed results broadly concur with Bagnold's criterion. Flow separation would be expected in the wide channel whereas the narrow channel is in the marginal region and although re-circulation is not observed, the inner bank velocities downstream of the apex are very small. In Figure 5 the numerical model is seen to have computed the water surface variation generally within ± 1 mm of the observed values.

4.3. Over-Bank Runs

The observed and calculated discharges in the main channel below floodplain level are shown in Table 2 below. For geometry 1 it can be seen that the main channel discharge at the apex (Q_{ca}) is well modelled except for run 121 where it is somewhat over-estimated. The source of this variation can be traced in more detail from Figures 6a-b. In Figure 6a-

b it is apparent that the maximum velocity filament in all flows is near the inside bank at the apex. For the lower bed slope (i.e. lower velocities) in Figure 6a the calculated and observed depth averaged velocity (DAV) values at the apex are in close agreement. However at the higher bed slope it can be seen from Figure 6b that a region of lower DAV values is observed at around 150mm from the outer bank.

The numerical model does not adequately capture this. The general circulation of the secondary flows at the apex is in the opposite sense to that observed for inbank flows and is captured in the model. This counter-rotation at the apex has been observed by a number of researchers and is driven by the channel/floodplain interaction at the adjacent upstream cross-over. It is seen that the observed rotation in the region of the low velocity zone is somewhat stronger than the modelled values.

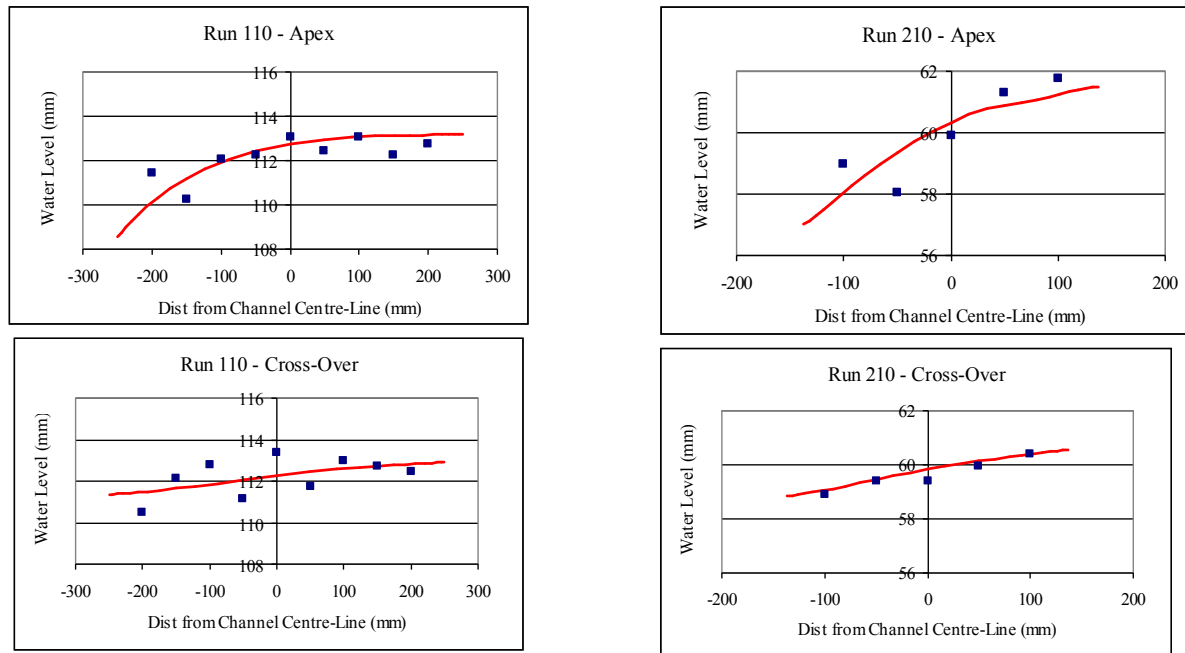


Figure 5. Water surface variations for inbank flow.

Table 2 – Comparison of Observed and Calculated Main Channel Flows

Run No.	Floodplain Bed Slope	Depth Ratio (H/h _c)	Total Flow (l/s)	Flows in Main Channel below Floodplain (l/s)			
				Observed		Calculated	
				Apex	Cross-Over	Apex	Cross-Over
Wide Channel – Planform 1							
110	0.00142	0.982	9.2	9.2		Input	Input
111	0.00142	1.383	25.6	12.1	10.7	12.07	10.13
112	0.00142	1.609	33.8	12.0	Not read	11.56	9.35
120	0.00498	0.991	14.7	14.7	14.7	Input	Input
121	0.00498	1.357	49.7	21.0	20.3	23.9	20.3
122	0.00498	1.670	78.0	25.6	22.8	24.8	19.9
Narrow Channel – Planform 2							
210	0.00175	0.937	4.4	4.4	4.4	Input	Input
211	0.00175	1.349	9.1	3.5	3.5	3.55	3.18
212	0.00175	1.778	20.1	4.5	4.0	4.26	3.62
230	0.00835	0.730	6.6	6.6	6.6	Input	Input
231	0.00835	1.365	19.0	7.9	7.6	7.68	6.46
232	0.00835	1.635	34.0	8.8	8.5	8.60	7.39

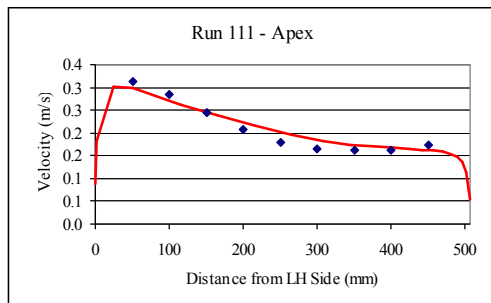
The cross-over DAV comparisons in Figure 6a indicate that the maximum velocity filament has moved across the channel from the inner bank at the apex to the outer bank at the cross-over, this is captured by the model. However at the higher bed slope in Figure 6b it is clear that the observed DAV values across the channel are more uniform laterally although the numerical model still gives higher DAV values at the outer bank. It is noticeable that the observed DAV at 100mm from the left hand for run

122 is lower than the adjacent points; this is not an error in reading and is repeated at other cross-over points. It is due to flow interference between the main channel and floodplain flows at the interface and results in the low DAV zone observed subsequently at the apex as discussed above.

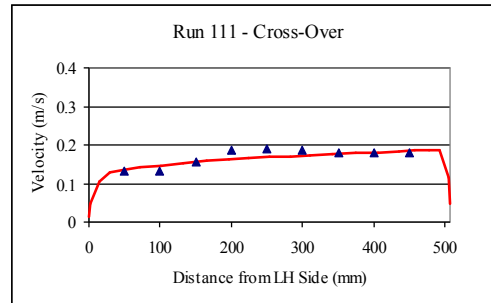
Figure 7 shows that the re-circulation zone observed in the in-bank flow does not occur in the over-bank flows. This re-circulation zone reduces the main channel capacity. As the flow is raised from in-

bank to low over-bank the main channel component will generally decrease due to turbulent energy losses at the main channel floodplain interface. Indeed this behaviour is seen for the narrow main channel in

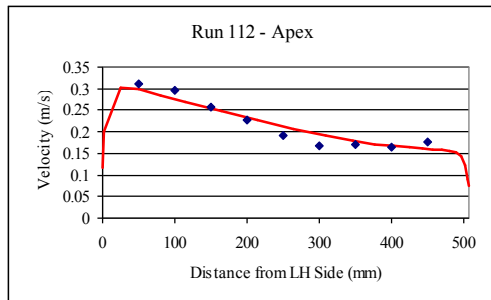
geometry 2. However, for geometry 1 the main channel component increases and this is due to the fact that in going over-bank the zone of re-circulation is removed.



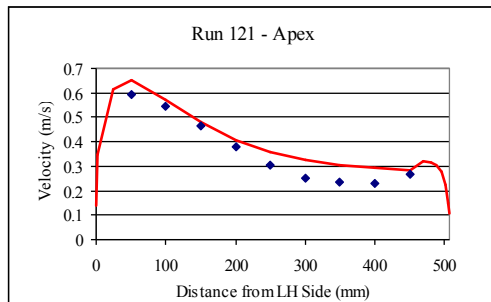
6a(i)



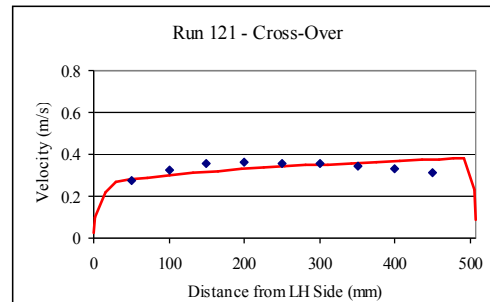
6a(ii)



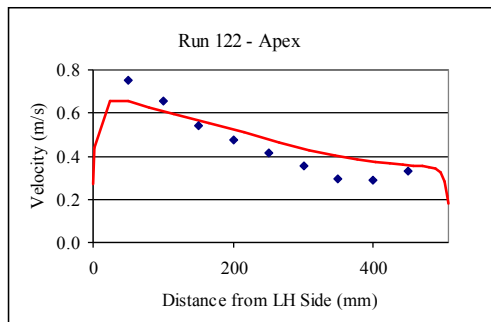
6a(iii)



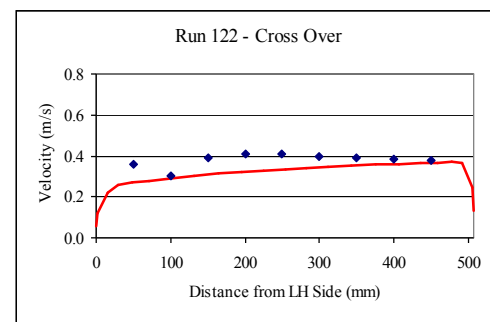
6b(i)



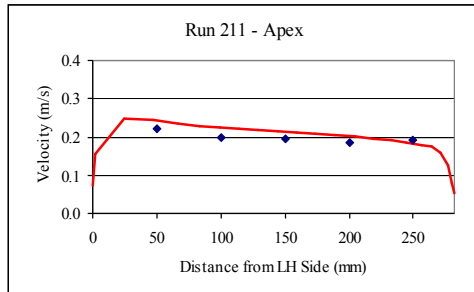
6b(ii)



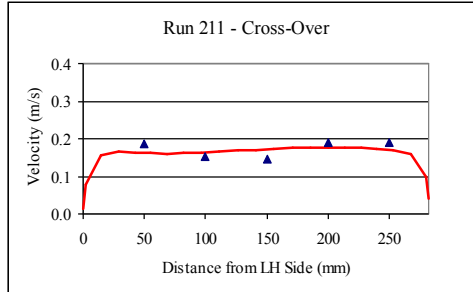
6b(iii)



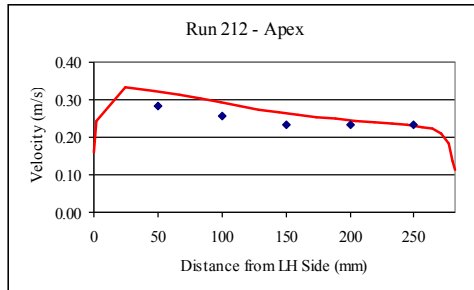
6b(iv)



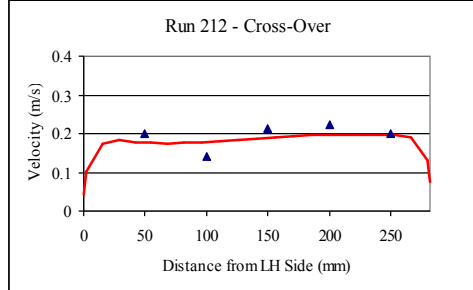
6c(i)



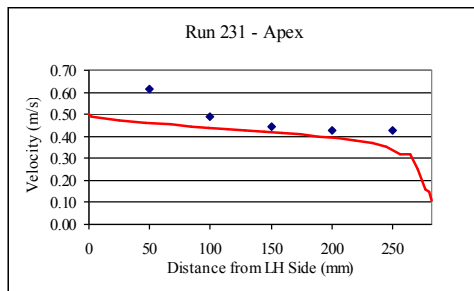
6c(ii)



6c(iii)



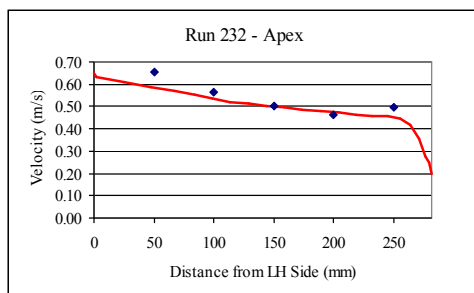
6c(iv)



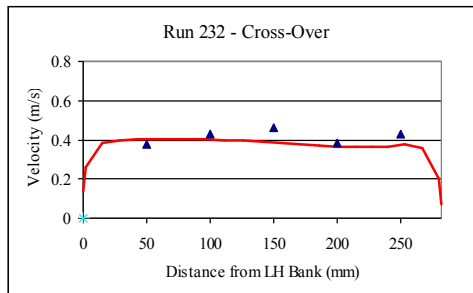
6d(i)



6d(ii)

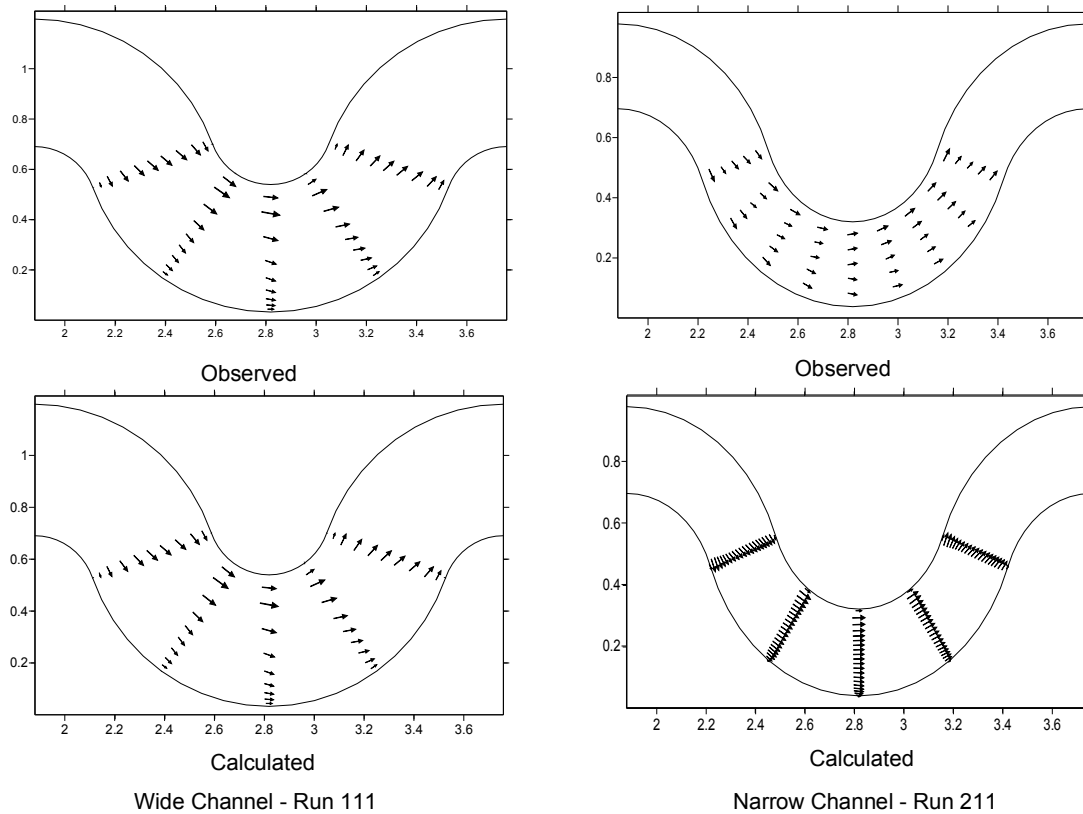


6d(iii)



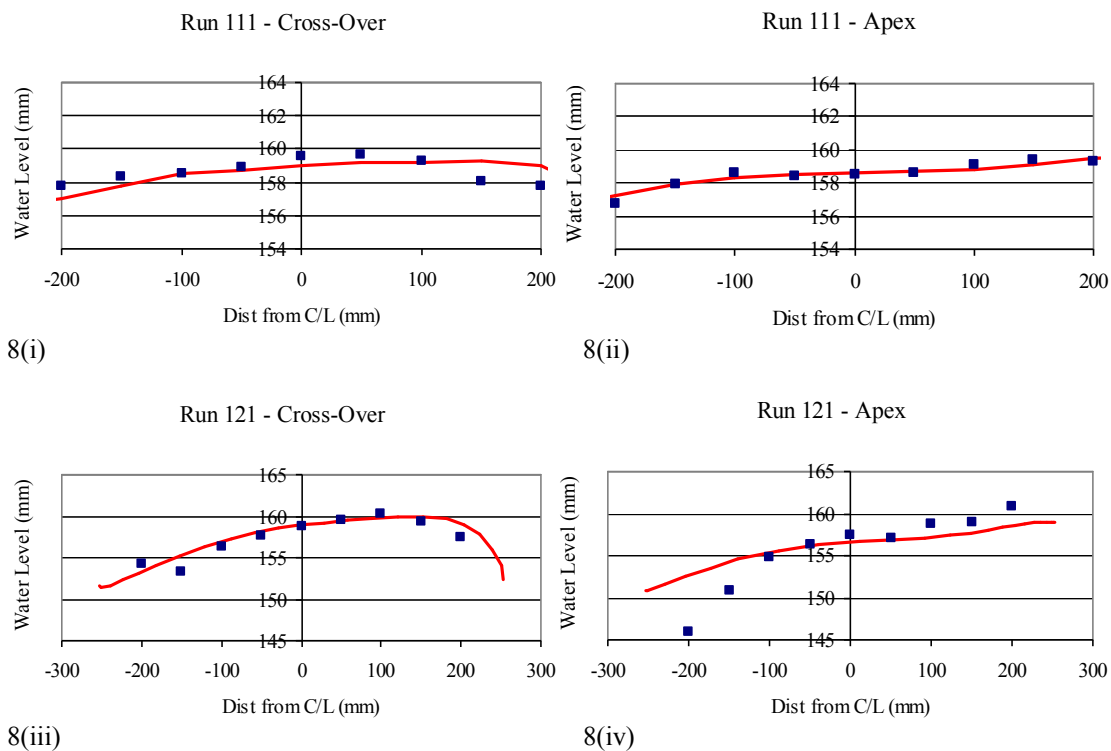
6d(iv)

Figure 6(a-d): Depth averaged velocities for low and high overbank flows at apex and cross-over for narrow and wide channels



Note: All horizontal scales are distance downstream in metres
 All vertical scales are lateral distance in metres.
 Velocity scale 0.3m/s = ↑

Figure 7. Planform velocity distributions for overbank flows - wide channel at 50mm above the bed and narrow channel at 45mm above the bed.



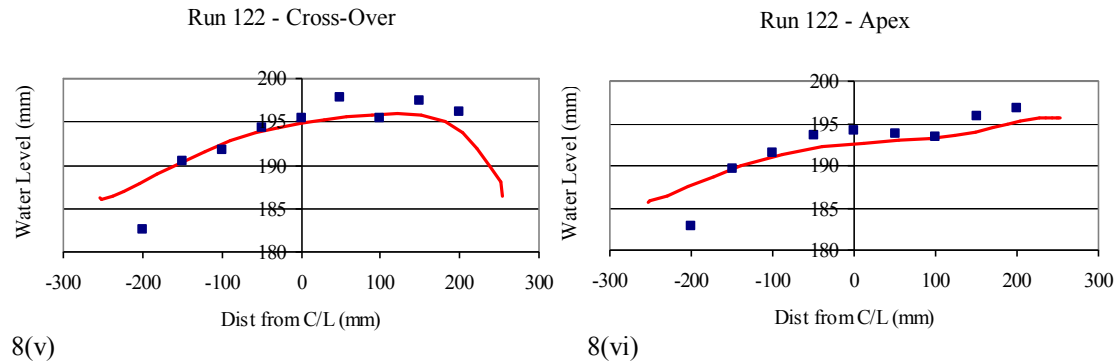


Figure 8. Water surface variation for overbank flows

It should be emphasised here that for the narrow channel geometry 2 only five lateral sets of readings were taken at 50mm spacing and at three depths of 20mm spacing. Thus there is some room for interpolation error between points and extrapolation error to the channel boundaries. Nevertheless it does appear that in general the apex flows are reasonably well modelled whereas the cross-over flows are underestimated. The geometry 2 DAV comparisons are shown in Figure 6c-d. It can be seen that the observed maximum DAV is again near the inside bank at the apices. The numerical model tends to over-estimate the maximum DAV for the lower bed slope as shown in Figure 6c and underestimate it at the higher flow and bed slope, run 232, as in Figure 6d. It is noticeable that the low velocity zone seen in the wider channel of geometry 1 does not occur in the narrower main channel. In fact a high velocity filament crosses from the outer bank at the upstream cross-over (-70°) to the inner bank halfway between the cross-over and the apex (-35°) where it remains until the next cross-over (70°). The numerical model captures this behaviour well leading to the relatively good predictions of discharge at the narrow main channel apices.

The cross-over discharges are underestimated increasingly with larger floodplain bed slope and depth of flow. In the experiments for the higher bed slope (231 and 232) surface standing waves were observed indicating flow at or near super-critical on the floodplain. This observation can be supported by considering the wide floodplain Froude numbers based upon Manning velocities. These are 0.69, 0.79, 1.66 and 1.82 for runs 211, 212, 231 and 232 respectively. At the cross-over points the floodplain flow crosses over the top of the main channel expanding as it enters the main channel and contracting as it leaves. If this process involves the transition from locally super-critical to sub-critical flow then a discontinuity (or standing wave) will occur. The numerical model handles such an abrupt

discontinuity by smoothing it out and this has indeed been observed in terms of calculated depth variations. The observed and calculated main channel depths for the steepest and deepest runs 231 and 232 are shown in Figure 8. The water surface levels in the numerical model agree well with observed values at the apex. However at the cross-over a standing wave some 12 mm high is observed and the numerical model simply smoothens this out. This may explain some of the relatively large discharge errors at the cross-overs, particularly since there is reasonable agreement between the observed and calculated DAV values as seen in Figure 6c-d.

5. Conclusions

In-bank and over-bank experimental data from a rectangular meandering channel at different bed slopes and flow depths have been modelled using a RANS k- ϵ and k- ω model. Two different main channel x-sections have been used with the same aspect ratio and having meander planform of the same sinuosity. Two different RANS k- ϵ methods discretisation grids were checked against each other, a structured grid using SSIIM and an unstructured one using FLUENT. Both gave very similar results confirming the grid independence of the model.

In-bank flows in the wider main channel (geometry 1) produced a recirculation zone near the inner bank downstream of the apex; this did not occur in the narrower main channel (geometry 2). The RANS k- ϵ model captured this recirculation zone although not its full extent. The k- ω gave very slightly improved results in the recirculation zone although it was almost identical elsewhere. The k- ω was shown to be faster and more robust, allowing larger relaxation factors. The in-bank water surface level variations were modelled satisfactorily.

The in-bank recirculation zone in geometry 1 did not appear for over-bank flows. This led to an increase in the main-channel discharge component as the flow went over-bank. For geometry 2 the main-

channel component initially decreased as the flow went over-bank, due to turbulent losses at the interface. The numerical model captured these different behaviours between the two geometries. The main-channel bulk discharge components for the over-bank flows at the apices were reasonably well predicted by the numerical model in all cases although those at the cross-overs were less so, particularly at higher depths and steeper bed slopes. This was thought to be due to the occurrence of standing waves on the floodplain caused by the floodplain flow crossing the main channel. The model tended to “smooth out” the discontinuities and rapid depth changes in these regions. The numerical model captured the general features of the observed data reasonably well. However it failed to capture a low velocity filament in the wide main channel which was created due to channel/floodplain interference at the upstream cross-over.

The simple k-e model has been tested against over-bank experimental data covering a range of depths, bed slopes and channel/floodplain geometries. It has been shown to provide reasonably good agreement in depth averaged velocities in all cases. It also appears to capture most of the overall salient flow structures. The RANS k-e model is one of the computationally fastest methods of modelling turbulent flows in open channel flows. It is particularly suited to meandering channels; both in-bank and over-bank, where the turbulent flows are largely geometry rather than boundary generated. As such it will be the method of choice in situations where the type/required accuracy of the output data, the channel geometry and the flow conditions permit. It is hoped that this paper will provide some guidance in identifying these conditions and limitations in the k-e model when applied to meandering over-bank flows.

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References

- 1 Ervine DA, Willetts BB, Sellin RHJ, Lorena M. Factors affecting conveyance in meandering compound flows. *Journal of Hydraulic Engineering* 1993; 119(12): 1383-99.
- 2 Wormleaton PR, Sellin RHJ, Loveless JH, Bryant T, Hey RD, Catmur SE. (2004). Flow structures in a two-stage meandering channel with a mobile bed. *Journal of Hydraulic Research* 2004; 42(2): 145-62.
- 3 Myers WRC, Lyness JE, Cassells JB, O'Sullivan JJ. Geometrical and roughness effects on compound channel resistance. *Proc. I.C.E. Water Maritime & Energy London*, 2000; 142(3): 157-66.
- 4 Wormleaton PR, Marriott MJ. An experimental and numerical study of the effects of width of meandering main channel on overbank flows. *IAHR Congress Venice Italy* 2007; 32: 575-82.
- 5 Wormleaton PR, Ewunetu M. Three dimensional k-ε numerical modelling of overbank flow in a mobile bed meandering channel with floodplains of different depth, roughness and planform. *Journal of Hydraulic Research* 2006; 44(1): 18-32.
- 6 Olsen NRB. *SSIIM Users' Manual*, The Norwegian University of Science and Technology, 2011.
- 7 Marriott MJ. The effect of overbank flow on the conveyance of the inbank zone in meandering compound channels. *IAHR Congress Graz Austria* 1999; 28: 612-18.
- 8 Marriott, MJ. Hydrodynamics of flow around bends in meandering and compound channels. Ph.D thesis, University of Hertfordshire, UK, 1998.
- 9 Patankar SV. *Numerical heat transfer and fluid flow*. McGraw-Hill, New York, 1980.
- 10 Bagnold RA. Some aspects of the shape of river meanders. *US Geological Survey, Professional Paper* 1960; 282E: 135-44.

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